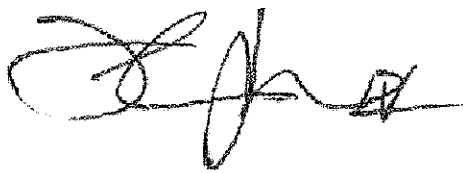


I HEREBY CERTIFY THAT THESE ENGINEERING DOCUMENTS WERE PREPARED BY ME OR UNDER MY DIRECT PERSONAL SUPERVISION AND THAT I AM A DULY LICENSED PROFESSIONAL ENGINEER UNDER THE LAWS OF THE STATE OF TEXAS.



07/29/2018

THOMAS POPELKA IV, P.E.

DATE

LICENSE NUMBER 88612

LICENSE RENEWAL DATE 06/31/2019

## LIMITS OF CERTIFICATION

- THE DESIGN OF THE STRUCTURAL WORK IS VALID ONLY FOR THIS SPECIFIC SITE. THE REUSE OF THIS DESIGN FOR OTHER BUILDINGS OR LOCATIONS IS STRICTLY PROHIBITED.
- SEALING OF THESE DRAWINGS DOES NOT IMPLY OR CONSTITUTE AND AGREEMENT BETWEEN THE SEALING ENGINEER WITH THE BUILDER, ARCHITECT, OR ANY ENTITY OTHER THAN THE CLIENT SPECIFICALLY NAMED ON THESE DRAWINGS. THE SEALING ENGINEER IS NOT ACTING AS THE "ENGINEER OF RECORD" FOR THIS PROJECT. FURTHERMORE, SEALING OF THESE DRAWINGS DOES NOT IMPLY RESPONSIBILITY FOR ANY AREA EXCEPT FOR THE STRUCTURAL DESIGN SPECIFICALLY SHOWN ON THESE DRAWINGS.
- THE ENGINEER'S STAMP ON THESE DRAWINGS IS CERTIFICATION THAT THE DESIGN OF THE FOUNDATION MEETS OR EXCEEDS THE MINIMUM STANDARDS OF THE BUILDING CODE.
- UPON COMPLETION OF THE PROJECT, AND UPON ACCEPTANCE OF ALL STRUCTURAL WORK, THE ENGINEER SHALL ISSUE A LETTER OF CERTIFICATION TO INDICATE WHETHER THE STRUCTURAL WORK CONFORMS TO THE DESIGN.

## INDEX TO DRAWINGS

S1.0	COVER SHEET & SPECIFICATIONS
S2.0	MAIN PLAN
S2.1	SHED PLAN
S3.0	ROOF PLAN
S3.1	DETAILS
S3.2	TRUSS PLAN AND DETAILS

## DESIGN LOADS

BUILDING CODE:	INTERNATIONAL BUILDING CODE; 2012 EDITION
USE CATEGORY:	II (NORMAL OCCUPANCY)
SNOW IMPORTANCE FACTOR (Is):	1.00
WIND IMPORTANCE FACTOR (Iw):	1.00
SEISMIC IMPORTANCE FACTOR (Is):	1.00
DEAD LOAD:	3.0 PSF (ESTIMATED)
LIVE LOAD (ROOF):	20.0 PSF, REDUCIBLE PER CODE PROVISIONS
SNOW LOAD:	5.0 PSF (Pg)
GCpi:	1.0
Ct:	1.0
WIND LOAD:	115 MPH; INLAND; EXPOSURE 'B'
Ce:	(+/-) 0.18
SEISMIC DATA:	
SHORT PERIOD ACCELERATION (Ss):	0.088
1-SECOND ACCELERATION (S1):	0.041
SEISMIC USE GROUP:	I
SOIL PROFILE TYPE:	E
SHORT SPECTRAL RESPONSE (SDs):	0.087
1-SECOND SPECTRAL RESPONSE (SD1):	0.059
SEISMIC DESIGN CATEGORY:	A
STRUCTURAL SYSTEM:	ORDINARY PLAIN MASONRY SHEAR WALLS
RESPONSE MODIFICATION FACTOR (R):	1.50
DEFLECTION AMPLIFICATION FACTOR (Cd):	1.50

## 1.0 CONTRACTOR RESPONSIBILITIES

- THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF THE WORK SHOWN ON THESE DRAWINGS AND VERIFICATION OF EXISTING CONDITIONS AT THE SITE. ANY DISCREPANCIES SHALL BE IMMEDIATELY REPORTED TO THE OWNER AND ENGINEER FOR RESOLUTION BEFORE PROCEEDING.
  - THE OWNER MAY DIRECT THE CONTRACTOR TO PERFORM ALTERNATE CONSTRUCTION WHERE REPAIR TO THE EXISTING STRUCTURE IS INDICATED IN THESE DRAWINGS.
- THE CONTRACTOR SHALL ASSURE THAT ALL CONSTRUCTION LOADS DO NOT EXCEED THE DESIGN LIVE LOADS SPECIFIED FOR THIS PROJECT.
- THE CONTRACTOR SHALL OBSERVE THE WORK IN PROCESS AND SHALL NOTIFY THE ENGINEER IN WRITING OF ANY LOADS NOT LISTED ABOVE. NO RESERVE LIVE LOAD CAPACITY EXISTS FOR CONVERSION TO OTHER PURPOSES.
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE SAFETY OF ALL CONSTRUCTION. COMPLETE CONSTRUCTION OF SHORING, BRACING, TIES, AND SUPPORTS SHALL BE USED TO ASSURE STRUCTURAL INTEGRITY DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL COMPLY WITH ALL OSHA STANDARDS, AND WITH ALL STATE AND LOCAL ORDINANCES.
- THE CONTRACTOR SHALL BEAR THE COST AND RESPONSIBILITY FOR OBTAINING ALL PERMITS REQUIRED FOR THE COMPLETION OF THIS WORK.

## 2.0 CONCRETE SPECIFICATIONS

- CONCRETE COLUMNS AND PEDESTALS SHALL BE REPAIRED IN STRICT CONFORMANCE WITH THESE SPECIFICATIONS.
- NON-SHRINK GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI WITHIN 3 DAYS. THE MAXIMUM EXPANSION SHALL BE 0.06%, AND SHALL HAVE A FINAL SET WITHIN 8 HOURS.
- CONCRETE GROUT SHALL HAVE A MIXING TEMPERATURE NOT TO EXCEED 90-DEGREES FAHRENHEIT.
- CUT OUT HONEYCOMB, ROCK POCKETS, VOIDS OVER 1/4" IN ANY DIMENSION, AND HOLES LEFT BY TIE RODS AND BOLTS, DOWN TO SOLID CONCRETE BUT, IN NO CASE TO A DEPTH LESS THAN 1-INCH. MAKE EDGE CUTS PERPENDICULAR TO THE CONCRETE SURFACE. THOROUGHLY CLEAN, DAMPEN WITH WATER, AND BRUSH COAT THE AREA TO BE PATCHED WITH AN APPROVED BONDING AGENT. PATCH WITH NON-SHRINK GROUT AFTER THE BONDING AGENT HAS DRIED.
- NO CONCRETE SHALL BE REPAIRED EXCEPT IN THE PRESENCE OF THE ENGINEER OR HIS REPRESENTATIVE.

## 3.0 WOOD FRAMING NOTES:

- WOOD AND TIMBER CONSTRUCTION SHALL CONFORM TO PROJECT SPECIFICATIONS AND AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) STANDARDS
- WOOD CONSTRUCTION SHALL CONFORM TO CHAPTER 23, OF THE INTERNATIONAL BUILDING CODE (UNLESS OTHERWISE NOTED). ALL NAILING SHALL CONFORM TO TABLE 2304.9.1 "FASTENING SCHEDULE" OF THE INTERNATIONAL BUILDING CODE UNLESS OTHER REQUIREMENTS NOTED ON THE PLAN ARE MORE STRICT. JOINTS IN LOAD BEARING TOP PLATES SHALL BE OFFSET AT LEAST 48". HOLES IN SAWN LUMBER SHALL BE AS SPECIFIED IN THE INTERNATIONAL BUILDING CODE.
- FRAMING LUMBER SHALL CONFORM WITH THE PROVISIONS OF THE AMERICAN SOFTWOOD LUMBER STANDARD PS-20-70 AND EACH PIECE SHALL BEAR THE GRADE STAMP OF A GRADING AGENCY APPROVED BY THE AMERICAN LUMBER STANDARDS COMMITTEE.
- DIMENSION LUMBER SHALL BE SOUTHERN PINE NUMBER 2 OR BETTER.
- NOTCHING OF JOISTS, RAFTERS, BEAMS OR TRUSSES IS NOT PERMITTED WITHOUT THE ENGINEER'S WRITTEN APPROVAL. HOLES BORED IN THE STUD OR JOIST SHALL BE IN THE MIDDLE ONE-THIRD OF THE DEPTH AND THE DIAMETER OF ANY SUCH HOLE SHALL NOT EXCEED ONE-FOURTH THE DEPTH.
- REPLACE ALL ROTTED, DAMAGED, SOFT, OR DETERIORATING WOOD JOISTS AND DECKING, WHETHER CALLED OUT ON THE DRAWINGS OR NOT.
  - NEW MATERIAL SHALL MATCH THE SIZE OF THE EXISTING MATERIAL.
  - EXISTING ROOF JOISTS THAT ARE CURRENTLY SPLICED SHALL BE REPLACED.
  - THE CONTRACTOR SHALL CONFIRM THE INTEGRITY OF THE EXISTING WOOD ROOF STRUCTURE BEFORE INSTALLING ROOFING OR OTHER CONSTRUCTION IN THE BUILDING.
- NEW WOOD DECKING SHALL SPAN ACROSS AT LEAST 3 ROOF JOISTS AND THE ORIGINAL WOOD DECKING SHALL ALSO SPAN ACROSS AT LEAST 3 ROOF JOISTS IN THE AREA OF REPAIR.
- NEW ROOF JOISTS SHALL SPAN FROM SUPPORT TO SUPPORT. SPLICED MATERIAL IS NOT PERMITTED.
- ALL EXISTING ROOF JOISTS SHALL BE ELEVATED TO A TRUE PLUMB CONDITION AND THEN REINFORCED WITH STEEL PLATE AS INDICATED IN THESE DRAWINGS.
- FASTENERS SHALL NOT BE LOCATED LESS THAN 1/2" IN FROM THE EDGE OF THE PANEL.
- FASTENERS SHALL BE DRIVEN FLUSH WITH SURFACE OF SHEATHING.
- FASTENERS SHALL BE OF SUFFICIENT LENGTH TO ENSURE PENETRATION INTO FRAMING MEMBERS BY AT LEAST 1".
- FRAMING MEMBERS SHALL BE A MINIMUM 2" NOMINAL IN THE DIMENSION TO WHICH THE STRUCTURAL PANEL IS ATTACHED.
- DECKING EDGES SHALL BUTT ALONG THE CENTERLINE OF FRAMING MEMBERS.
- FASTENERS SHALL BE A MINIMUM 8d (.131") COMMON OR GALVANIZED BOX NAILS (GALVANIZED NAILS SHALL BE HOT-DIPPED OR TUMBLED).
- OFFSET ALL DECKING JOINTS A MINIMUM ONE JOIST BAY.

## 4.0 STEEL SPECIFICATIONS

- REPLACE OR REPAIR STEEL FRAMING AND CONNECTIONS AS INDICATED ON THE DRAWINGS.
  - ALL NEW STEEL FRAMING SHALL MATCH THE EXISTING FRAMING THAT IS REPLACED.
  - ALL NEW BOLTS SHALL BE THE SAME SIZE AS THE RIVETS OR BOLTS THAT ARE REPLACED.
- STRUCTURAL STEEL SHALL BE FABRICATED TO FIT ACTUAL SITE DIMENSIONS. ROUGH DIMENSIONS ARE PROVIDED ON THE DRAWINGS, BUT THE CONTRACTOR SHALL BE RESPONSIBLE FOR TRUE FIT-UP OF ALL STRUCTURAL ELEMENTS.
  - STEEL FRAMING MAY BE EITHER FIELD-FABRICATED OR SHOP FABRICATED.
  - ALL STEEL FRAMING SHALL BE SAW CUT. TORCH CUTTING IS NOT PERMITTED.
- STEEL FABRICATION SHALL CONFORM TO AISC SPECIFICATIONS AND TOLERANCES. THESE INCLUDE, BUT ARE NOT LIMITED TO:
  - HOLES SHALL BE DRILLED OR PUNCHED. TORCH-CUT HOLES ARE NOT PERMITTED.
  - STEEL SHALL CONFORM TO THE AISC TOLERANCES FOR CAMBER AN SWEEP.
- MATERIAL PROPERTIES OF STEEL FRAMING SHALL BE AS FOLLOWS:
  - ALL STEEL PLATE SHALL CONFORM TO ASTM A572 OR A992, AND SHALL HAVE A MINIMUM YIELD STRENGTH OF 50KSI.
  - ALL HOT-ROLLED STEEL ANGLE AND CHANNEL SHALL CONFORM TO ASTM A36, AND HAVE A MINIMUM YIELD STRENGTH OF 36KSI.
- EXISTING RIVETS OR BOLTS THAT ARE REPLACED SHALL BE CUT BY GRINDING.
- ALL NEW BOLTS SHALL BE ASTM A325 UNLESS NOTED OTHERWISE.
- WHERE EXISTING FRAMING MATERIAL IS STRAIGHTENED BY USING HEAT, THE CONTRACTOR SHALL PROTECT ALL ADJACENT WOOD SURFACES AND SHALL HAVE ALL BE RESPONSIBLE FOR OBTAINING ALL PERMITS THAT ARE REQUIRED FOR THIS WORK.
- NEW ANCHORS SHALL BE ASTM A307 AND SHALL BE INSTALLED WITH EPOXY ADHESIVE.
  - THE NEED FOR NEW ANCHORS SHALL BE DETERMINED AT THE SOLE DISCRETION OF THE ENGINEER.
  - EPOXY ADHESIVE SHALL BE RESI-WELD BY W.R. MEADOWS, OR EQUAL.

## 5.0 DEMOLITION

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFE DEMOLITION AND REMOVAL OF THE STEEL STORAGE STRUCTURE AT THE NORTH SIDE OF THE MAIN BUILDING.
- THE CONTRACTOR SHALL AT ALL TIMES PROTECT THE STRUCTURE OF THE MAIN BUILDING FROM THE COLLAPSING STEEL STRUCTURE OR OTHER FALLING DEBRIS.
- AFTER REMOVAL OF THE POSTS, THE CONTRACTOR SHALL ALSO REMOVE THE POST FOOTINGS IF POSSIBLE. IF THE POST FOOTINGS SHALL REMAIN, THEN THE SURFACE AREA AT THE FOOTING SHALL BE REPAIRED TO MATCH THE EXISTING ADJACENT SURFACE.
  - WHETHER POST FOOTINGS SHALL REMAIN SHALL BE THE SOLE DECISION OF THE OWNER UPON ADVISE OF THE OWNER'S REPRESENTATIVE ENGINEER.

## 6.0 SUBMITTALS

THE CONTRACTOR SHALL SUBMIT THE FOLLOWING WRITTEN REPORTS TO THE ENGINEER TO CONFIRM COMPLIANCE WITH THE SPECIFICATIONS AND THE QUALITY ASSURANCE PROGRAM.

- THE CONTRACTOR SHALL SUBMIT MATERIAL DATA SHEETS FOR NON-SHRINK GROUT.
- THE CONTRACTOR SHALL SUBMIT MILL BATCH REPORT FOR ALL STEEL FRAMING.
- THE CONTRACTOR SHALL SUBMIT SAMPLES OF BOLTS.
- THE CONTRACTOR SHALL SUBMIT MATERIAL DATA SHEETS FOR ALL LUMBER.

## 7.0 INSPECTIONS

- IN CONFORMANCE WITH OR IN ADDITION TO THE QUALITY ASSURANCE PROGRAM, INSPECTIONS SHALL BE SCHEDULED TO OBSERVE THE WORK AS DESCRIBED IN THIS SECTION.
- SUBMITTAL AND INSPECTION REQUIREMENTS SHALL BE STRICTLY ENFORCED. THE CONTRACTOR'S FAILURE TO PERFORM SHALL BE CAUSE FOR REJECTION OF THE WORK.
- THE CONTRACTOR SHALL PROVIDE A PROJECT SCHEDULE TO THE ENGINEER AND THE INSPECTOR, INCLUDING TIMELY UPDATES, TO COORDINATE THE SCHEDULING OF INSPECTIONS. THE ENGINEER AND INSPECTOR SHALL BE NOTIFIED AT LEAST 48 HOURS IN ADVANCE OF WORK THAT SHALL BE READY TO INSPECT.
- STRUCTURAL WORK THAT SHALL BE INSPECTED SHALL NOT BE COVERED BEFORE THE WORK IS INSPECTED AND ACCEPTED. STRUCTURAL WORK THAT HAS BEEN INSPECTED SHALL NOT BE ALTERED AFTER THE INSPECTION UNLESS AUTHORIZED BY THE ENGINEER IN WRITING.
- WHERE CONCRETE IS REPAIRED, THE ENGINEER OR HIS REPRESENTATIVE WILL INSPECT THE CONCRETE PREPARATION BEFORE NEW CONCRETE GROUT IS APPLIED.
- A FINAL INSPECTION OF THE PLACED CONCRETE, AFTER FORMWORK IS REMOVED, SHALL BE PERFORMED BY THE ENGINEER OR HIS REPRESENTATIVE.
- THE STEEL FRAMING SHALL BE INSPECTED WHEN THE WORK STEEL REPAIR WORK IS COMPLETE.
- THE WOOD FRAMING AND DECKING SHALL BE INSPECTED BEFORE ROOFING IS INSTALLED.

### General Notes

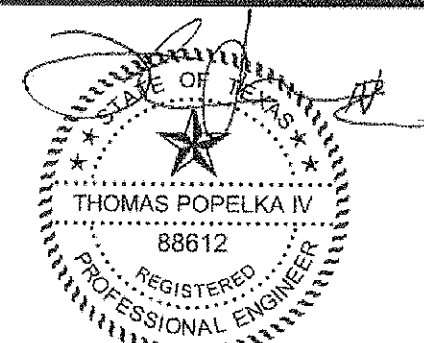
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No.	Revision/Issue	Date

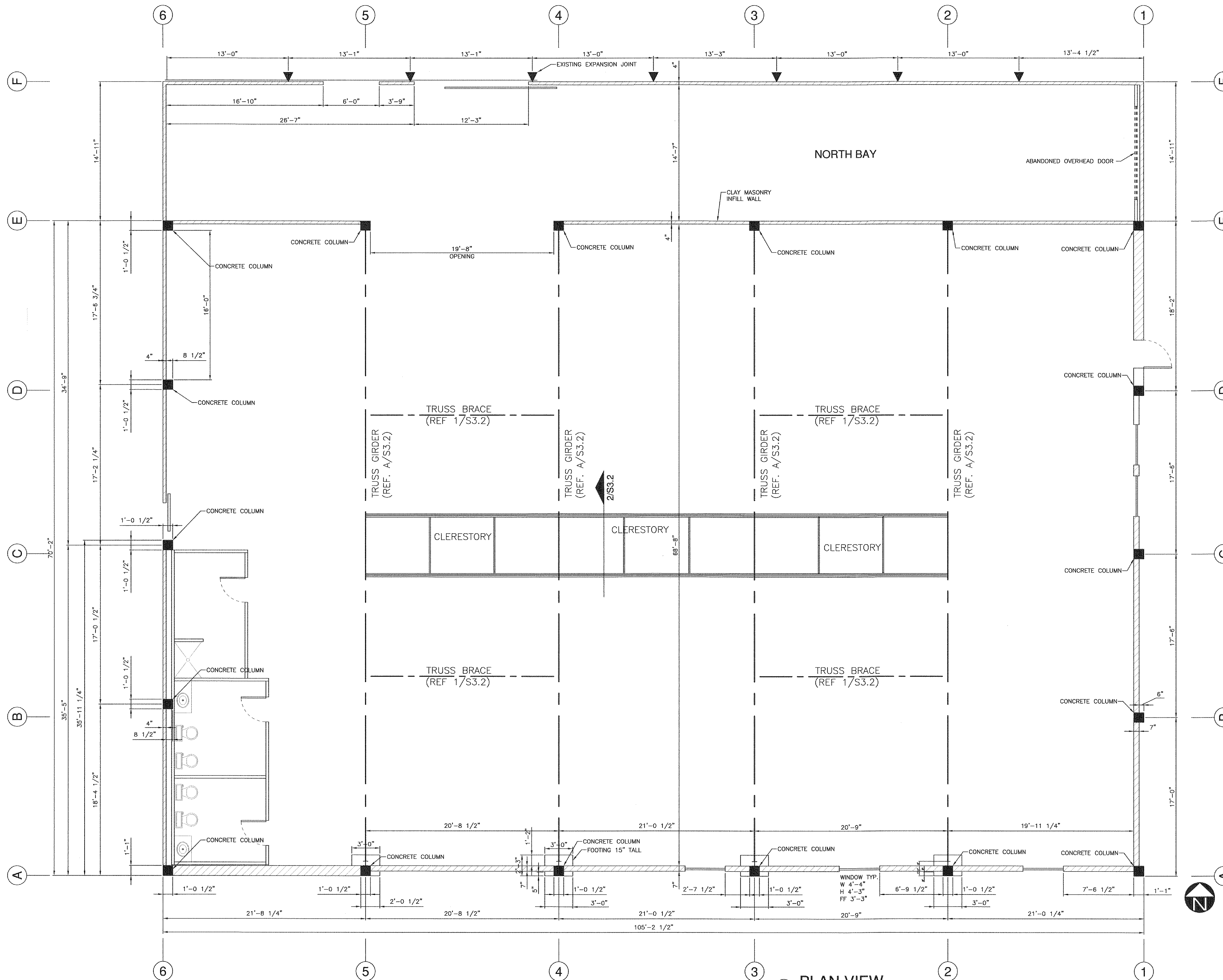


DATE: 07/29/2018

POPELKA STRUCTURAL ENGINEERING  
2406 BROOKLAWN DRIVE  
TEMPLE, TX 76502  
(254) 773-8882

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214 SOUTH 2nd  
TEMPLE, TEXAS 76502  
Drawings prepared for:  
TEMPLE CHILDREN'S MUSEUM  
214 SOUTH 2nd  
TEMPLE, TEXAS

Project 0180032	Sheet
Date 07/29/2018	S1.0
Scale SCALE	



#### General Notes

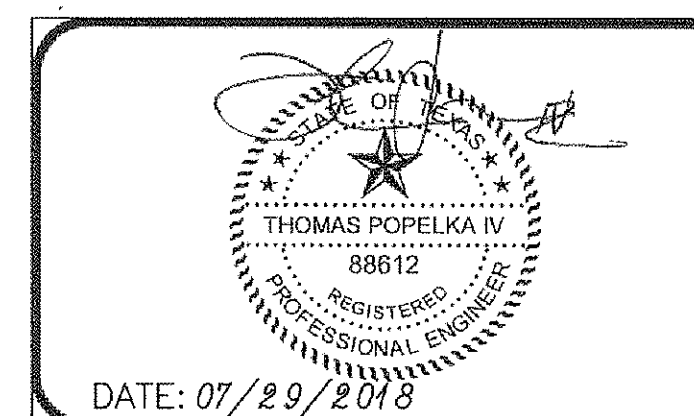
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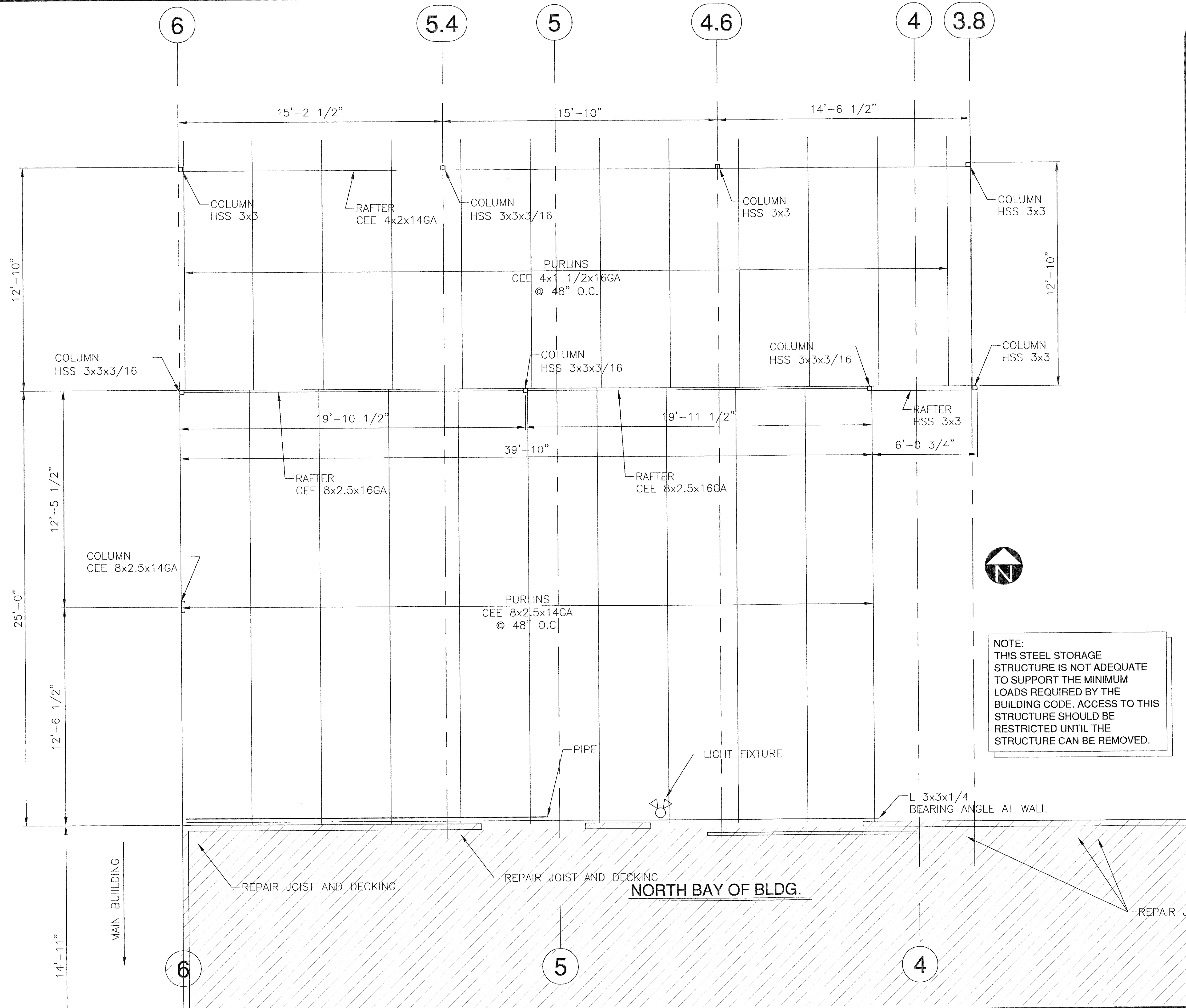
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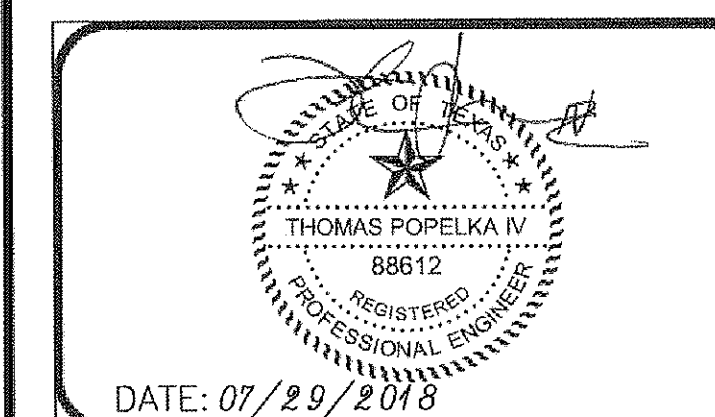
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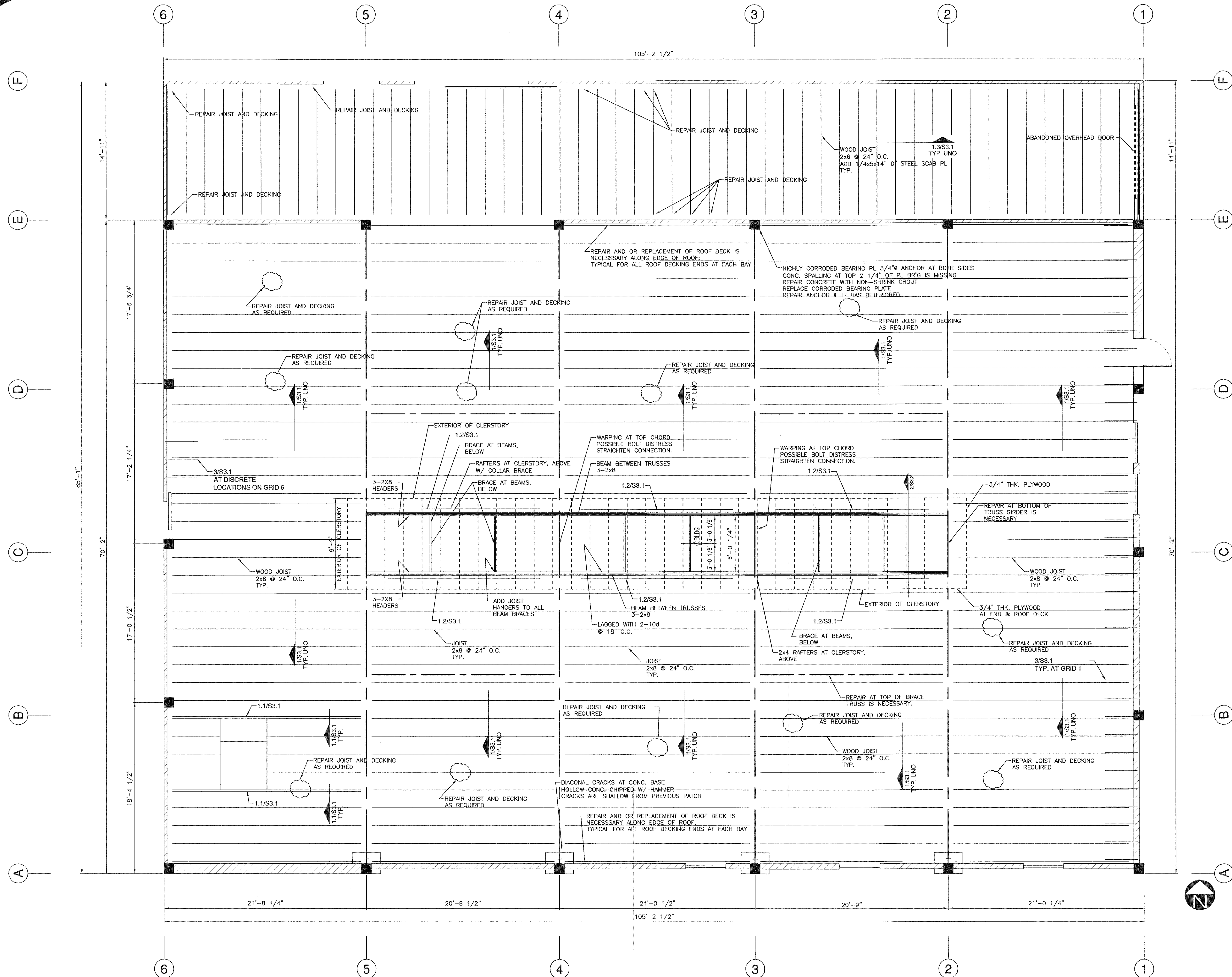



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S2.1



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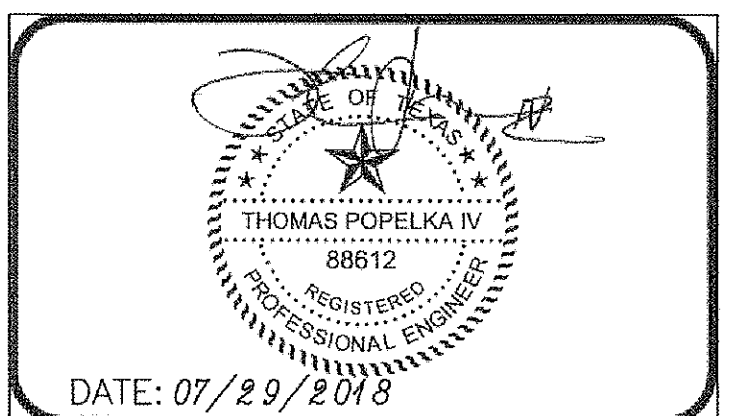
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S3.0

(2) 1/4" WOOD SCREW  
AT 12" O.C.

PL 1/8x8x18'-4"  
REINFORCING PLATE  
(PRE-DRILL)

EXISTING WOOD DECK

EXISTING 2x8 ROOF JOIST

## 1 SECTION AT ROOF JOIST

3/4"=1'-0"  
REFER TO DETAIL 2/S3.1

(2) 1/4" WOOD SCREW  
AT 12" O.C.

PL 1/4x8x16'-4"  
REINFORCING PLATE  
(PRE-DRILL)

EXISTING WOOD DECK

EXISTING 2x8 ROOF JOIST

## 1.1 REINFORCING AT ROOF HATCH

3/4"=1'-0"  
REFER TO DETAIL 2/S3.1 (SIM)

(2) 1/4" WOOD SCREW  
AT 12" O.C.

PL 3/8x8x16'-0"  
REINFORCING PLATE  
(PRE-DRILL)

EXISTING WOOD DECK

EXISTING 3-2x8 CLERESTORY BEAMS

## 1.2 REINFORCING AT CLERESTORY BEAMS

3/4"=1'-0"  
REFER TO DETAIL 2/S3.1

(2) 1/4" WOOD SCREW  
AT 12" O.C.

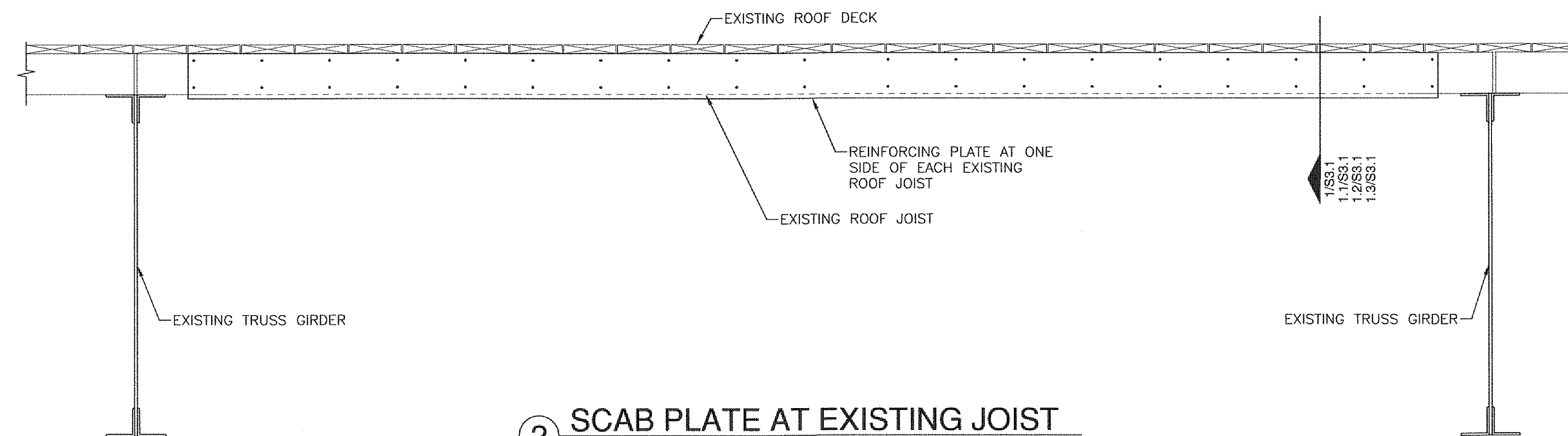
PL 1/4x5x14'-0"  
REINFORCING PLATE  
(PRE-DRILL)

EXISTING WOOD DECK

EXISTING 2x6 ROOF JOIST

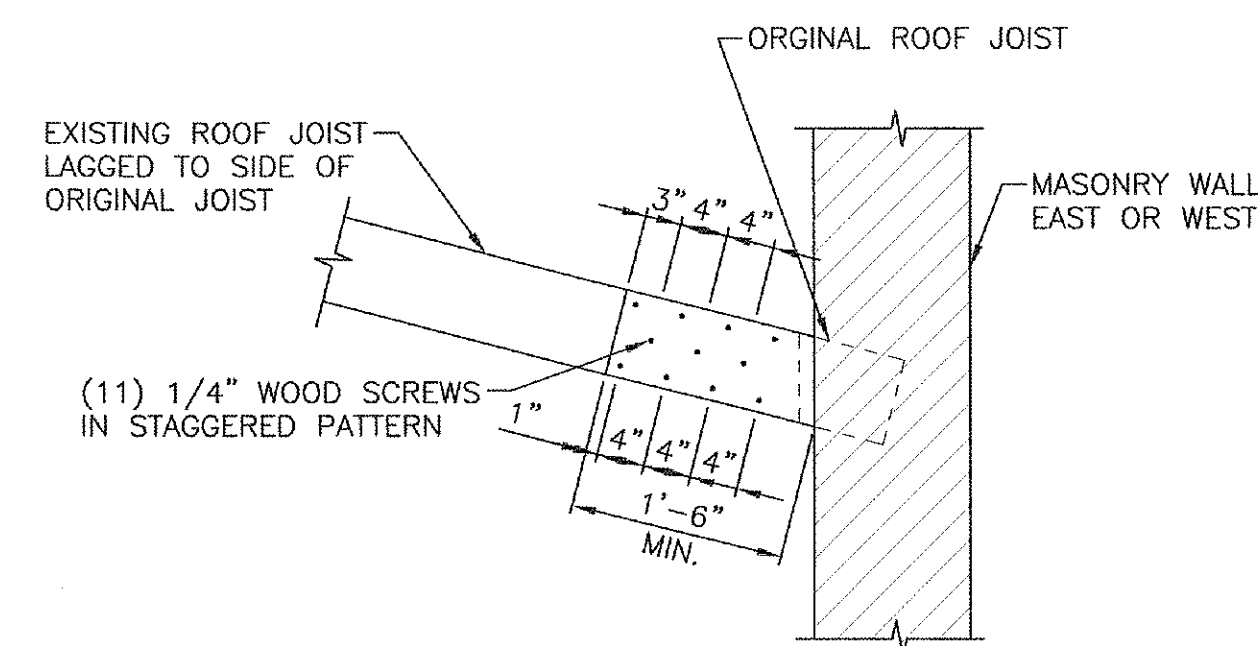
## 1.3 REINFORCING AT LEAN-TO

3/4"=1'-0"  
REFER TO DETAIL 2/S3.1 (SIM)



## 2 SCAB PLATE AT EXISTING JOIST

3/4"=1'-0"  
NOTE: PLUMB THE JOIST TO REMOVE SAGGING BEFORE  
INSTALLING THE REINFORCING PLATE.  
REINFORCING IS REQUIRED AT ALL JOISTS



## 3 CONNECTION AT LAGGED ROOF JOISTS AT FRONT AND BACK WALLS

3/4"=1'-0"

### General Notes

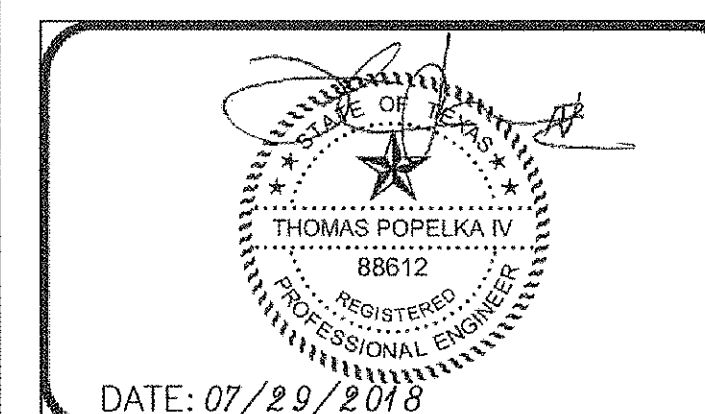
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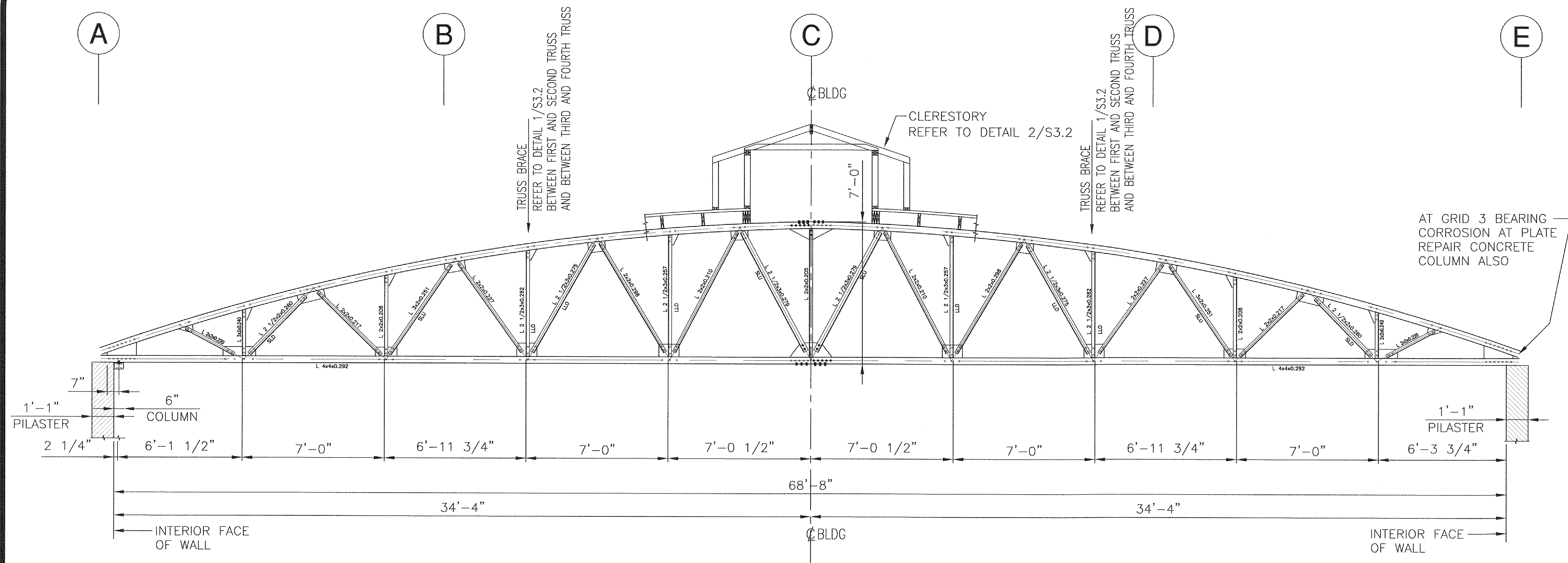
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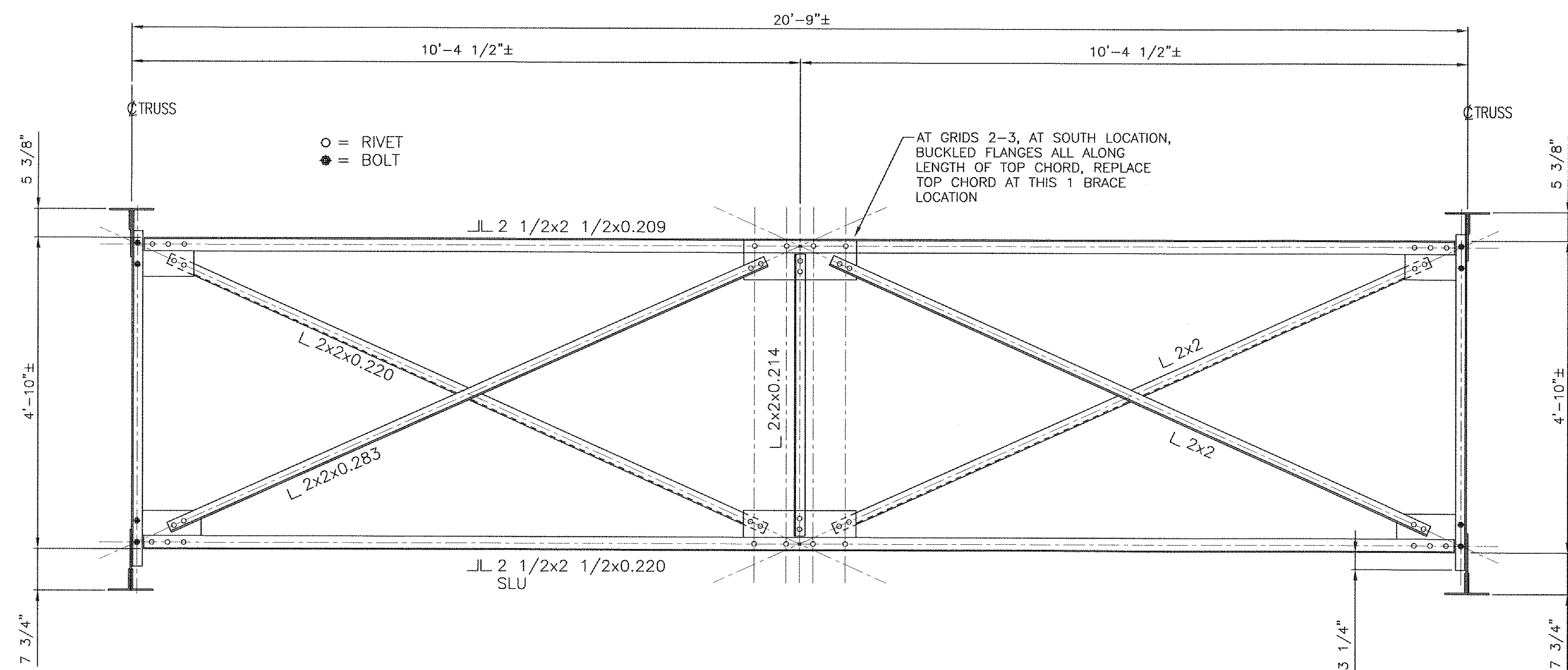
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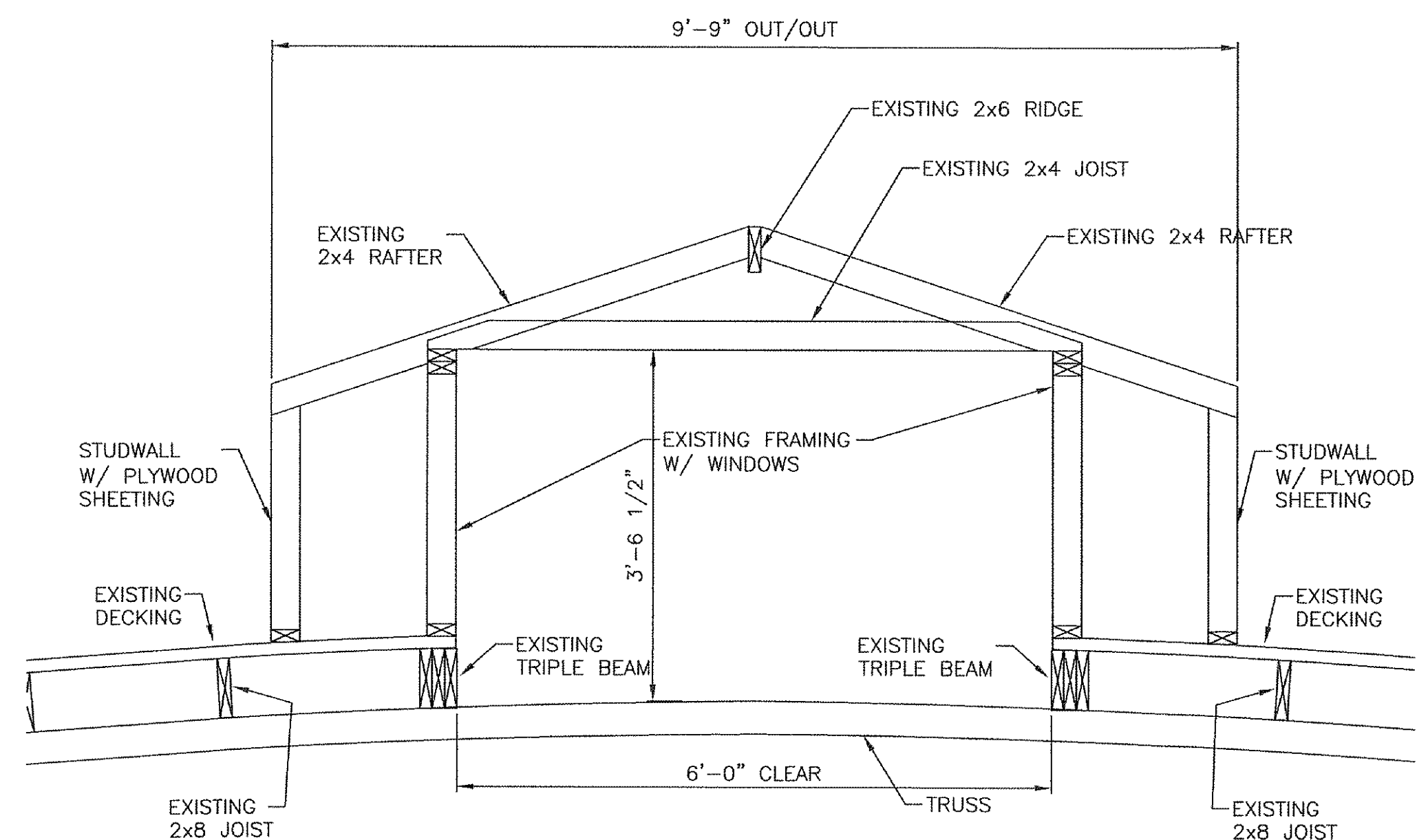
Project 0180032	Sheet <b>S3.1</b>
Date 07/29/2018	
Scale 3/4"=1'-0"	



**A MAIN TRUSS GIRDER AT GRIDS 2, 3, 4, & 5**  
3/8"=1'-0"



**1 TRUSS GIRDER BRACING**  
3/4"=1'-0"



**2 CLERESTORY AT TOP OF TRUSS**  
3/4"=1'-0"

General Notes

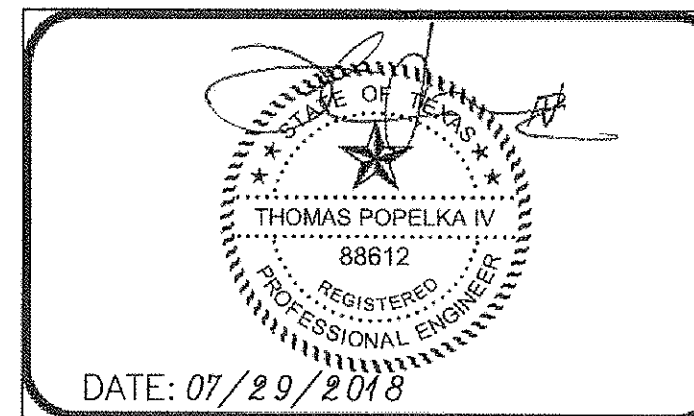
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**S3.2**